

"20"  
1/10

# Hydraulics

3<sup>rd</sup> Year civil

First Term (2009 - 2010)

Chapter ( )

**Revision Part (2)**  
**final 2005**

Solve as much as you can. (N.B. Any missing data can be reasonably assumed).

**Question 1-**

- A- Sketch the velocity distribution over a smooth channel surface, comment very briefly.
- B- Plot the isovels for very wide and very narrow rectangular channel and explain how the boundary resistance affects on the velocity distributions, then sketch samples of verticals and lateral velocity distributions for each one.
- C- Draw the relationship between  $C$ , and  $n$  when the hydraulic radius is constant.
- D- Sketch the hydraulic jump interpreted by specific energy and specific force diagrams, express and show few of the hydraulic jump characteristics.
- E- What is the minimum slope at which  $28 \text{ m}^3/\text{sec}$  may be carried out uniformly at a mean velocity of  $0.6 \text{ m/sec}$  in a trapezoidal canal having  $n=0.025$  and side slopes 1 (vertical): 2 (Horizontal)? Redesign for hydraulically stable section, if the maximum velocity is  $0.4 \text{ m/s}$ ,  $d_{50} = 0.15 \text{ mm}$ .

**Question 2-**

- A- Discuss very briefly about:  
Control volume, Control section, State of flow, regime of flow.
- B- Demonstrate the effect of a sudden rise in the bed of an open channel Applying the specific energy analysis for different approaching flow conditions, showing the limiting case.
- C- Plot  $b/P$  versus the slope of sides for trapezoidal channel in case of best hydraulic section in which  $b$  is the bed width and  $p$  is the wetted perimeter. Sketch  $\tau_0$  distribution for a selected case.
- D- A hydraulic jump was formed in a rectangular open channel of width  $4.0 \text{ m}$ , has its sides expanding  $0.3 \text{ m}$  from each side at a given section, if the two conjugate depths are  $0.50 \text{ m}$  and  $6.0 \text{ m}$  respectively. And the depth of water downstream the contracted part is  $4.50 \text{ m}$ . calculate the ~~discharge~~  $Q$  passing, power dissipated by the jump in  $\text{K}$ ,  $\text{wt}$ , and the overall efficiency of the hydraulic jump.

**Question 3-**

- A- Classify the water surface profiles according to the bed slope, give practical examples for each.
- B- What is meant by "stilling basin" using a sketch, explain how to control the location of the jump in an open channel.
- C- A uniform flow occurs in a very long rectangular channel, the normal water depth is  $0.9 \text{ m}$  and the mean velocity is  $4.8 \text{ m/sec}$ . A vertical sluice gate is constructed and lowered so that the opening is  $0.60 \text{ m}$ . sketch the new water surface profiles to be expected and identify them by letter and number. Calculate and show all significant depths and expected T.E.L. noting that the normal water depth D.S. the gate exert when the mean velocity equals to  $0.8 \text{ m/s}$ . (assuming  $n=0.025$ ).

#### Question 4-

- A- State the advantages and disadvantages of hydraulic modeling.
- B- What is meant by "complete dynamic similarity" for hydraulic modeling? Give one practical example.
- C- Select any hydraulic phenomenon then write all forces affecting upon this phenomenon, deduce all dimensionless groups which controlling it, and show how the selected dimensionless parameters are correlated.
- D- Drive an expression for Manning coeff. ratio  $n_r$ , Chezy coeff. ratio  $C_r$  and shear velocity ratio  $U_r$  for open channel flow, according to certain scale ratio.

#### Question 5-

- A- If the energy loss through a channel reach can be neglected, what type of flow would be expected?
- B- State why the dimensional analysis is necessary?
- C- List the different practical methods used to measure flow rates in open channels and very wide rivers? derive an expression for critical depth meter?
- D- A model was constructed with scale ratio 1:20 to study the problem of energy dissipater. By means of theory of least squares determine the constant in:  $\frac{y_2}{y_1} = aF_1 + b$  for the following results.

$Y_1$ (cm)	1.44	2.25	3.47	3.99
$Y_2$ (cm)	9.81	10.89	17.92	19.99
$Q$ (lit/sec)	5.9	7.48	16.31	19.51

If the rectangular channel width = 0.3 m, then find the relative depth of hydraulic jump at  $Q=18.1$  Lit/sec and  $y_1=2.4$  cm.

- E- find the discharge passing in the prototype at inflow Froude number = 3.0 and  $y_1=3.6$  cm, then find the sequent depth of the jump.
- F- Using the dimensional analysis, correlate all involved variables affecting such phenomenon..

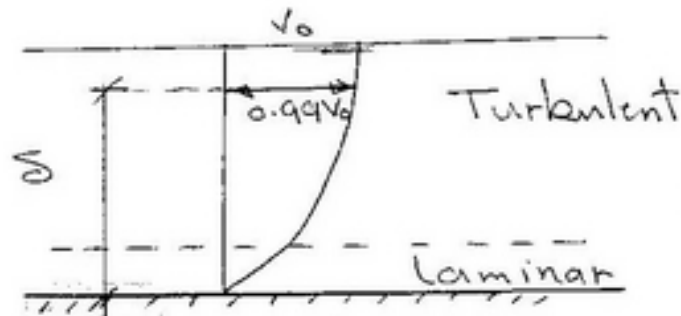


بِسْمِ اللَّهِ الرَّحْمَنِ الرَّحِيمِ

Final 2005

Q (1) :

(A) :

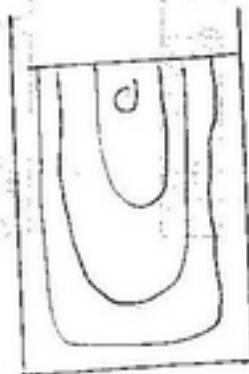


بالقرب منه قاع البحر إلى ينزل السريانه سراعاً نظراً  
لأنه خفاخف فيه السريه . وكلما ابتعدنا ازداد السريه  
ويطول السريانه إلى سريانه مضطرب

(B) :

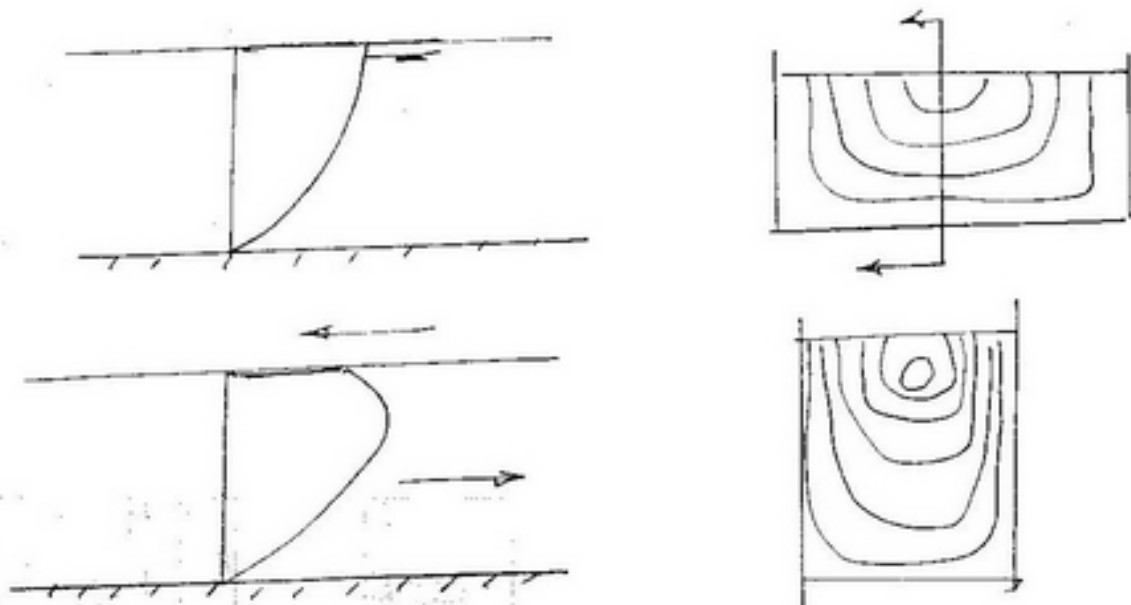
v. deep  
v. narrow

y. wide  
y. shallow

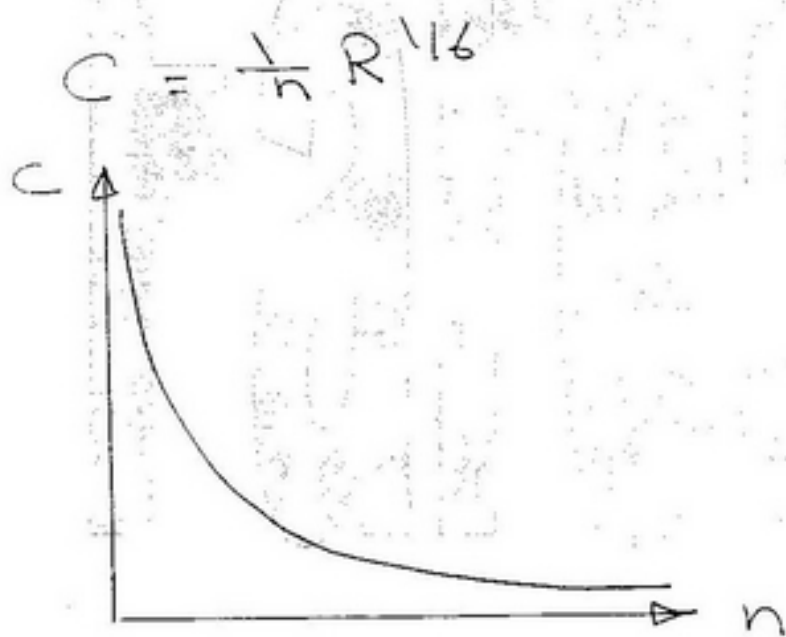


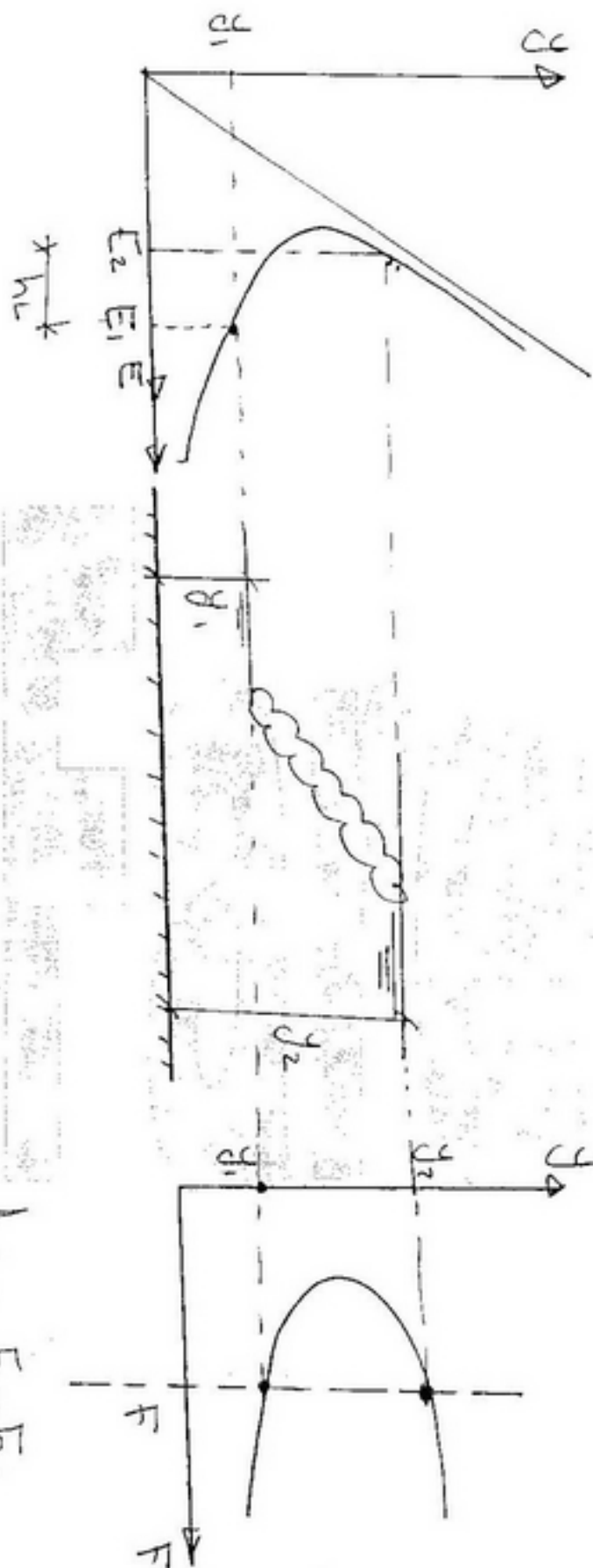
مركز البحوث والدراسات  
الاسلامية  
الاسلامية  
الاسلامية

۱- اگر احتمال بجواند اجرای لای تقاضای میده، سریع  
مکملات را بپردازد و بعد از اجرای لای کل تأثیرات  
مزاد آن میده، سریع.



(C):





$$H_j = y_2 - y_1, \quad L_j = 5.2 y_2, \quad h_L = E_1 - E_2$$

$$z = \frac{E_2}{E_1}, \quad \frac{y_2}{y_1} = 0.5 \left[ \sqrt{1 + 8 F_1^2} - 1 \right]$$

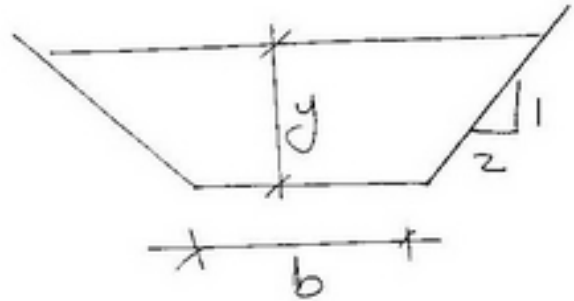
(D):

(E) :

-  $Q = 28 \text{ m}^3/\text{s}$

-  $V = 0.60 \text{ m/s}$

-  $n = 0.025$



Req. (a) find min. slope

(b) Redesign section

$V = 0.4 \text{ m/s}$  also  $S_o = 1.5 \text{ mm}$

Sol. :

$$\therefore Q = \frac{1}{n} \cdot \frac{A^{5/3}}{P^{2/3}} \cdot S^{1/2}$$

for min. slope B.H.S

$$R = \frac{y}{2} = \frac{A}{P}$$

$$A = (b + zy)y = (b + 2y)y$$

$$P = b + 2y\sqrt{1+z^2} = b + 4.47y$$

$$\therefore \frac{y}{2} = \frac{(b+2y)y}{b+4.47y}$$

$$b + 4.47y = 2b + 4y$$

$$b = 0.47y \rightarrow \textcircled{1}$$

$$\therefore A = \frac{Q}{V} = \frac{28}{0.6} = 46.7 \text{ m}^2$$

$$46.7 = (b + 2y)y \rightarrow \textcircled{2}$$

From  $\textcircled{1}$  in  $\textcircled{2}$

$$46.7 = (0.47y + 2y)y$$

$$46.7 = 2.47y^2$$

$$y = 4.35 \text{ m}$$

$$b = 2.05 \text{ m}$$

$$P = 2.05 + 4.47 \times 4.35 = 21.5 \text{ m}$$

$$\therefore 28 = \frac{1}{0.025} \times \frac{(46.7)^{5/3}}{(21.5)^{2/3}} \times S^{1/2}$$

$$S = 0.0001 \quad \#$$



$$Q = 28 \text{ m}^3/\text{s}$$

$$V = 0.4 \text{ m/s}$$

$$d_{50} = 1.5 \text{ mm}$$

$$S = 0.0001$$



assume  $b/y = 6$

$$\tau_s = 0.75 \times y \times S$$

$$\tau_b = 0.97 \times y \times S$$

$$\ll \tau_{cr}$$

$$\tau_{cr} = 0.15 \text{ kg/m}^2$$

$$\therefore 0.15 = 0.75 \times 1000 \times y \times 0.0001$$

$$y = 2.0 \text{ m}$$

$$0.15 = 0.97 \times 1000 \times y \times 0.0001$$

$$y = 1.55 \text{ m}$$

$$y = 1.55 \text{ m}$$

$$b = 9.3 \text{ m}$$

$$\therefore Q = A \times V$$

$$A = (9.3 + 2 \times 1.55) \times 1.55 = 19.2$$

$$Q = 19.2 \times 0.4 = 7.70 \text{ m}^3/\text{s} < 28$$

assume  $\frac{b}{y} = 10$

$$y = 1.55, \quad b = 15.5 \text{ m}$$

$$A = (15.5 + 2 \times 1.55) \times 1.55$$

$$A = 28.8$$

$$Q = 0.4 \times 28.8 < 28$$

فرض

$$\therefore Q = A \times V$$

$$28 = A \times 0.4$$

$$A = 70 \text{ m}^2$$

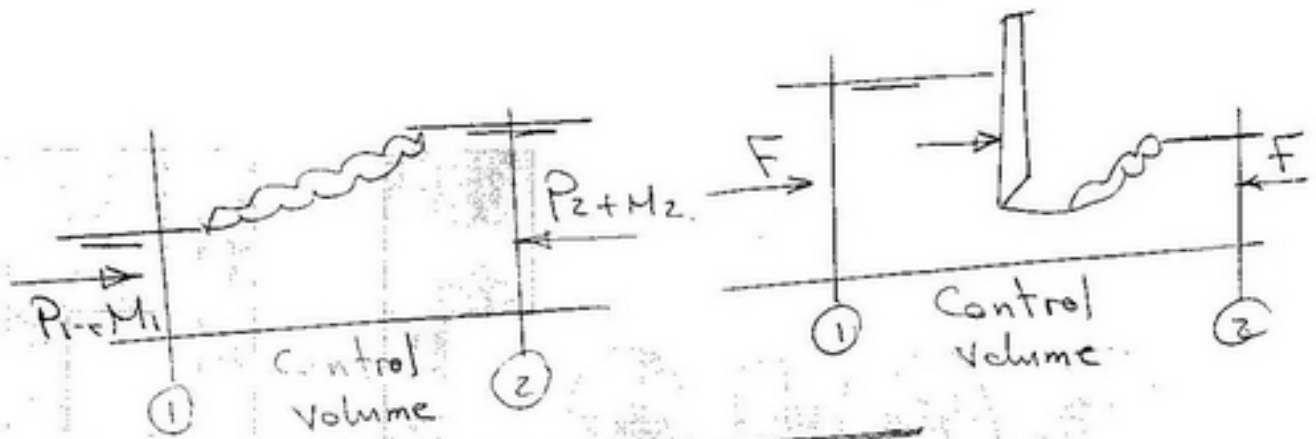
$$70 = (b + 2 \times 1.55) \times 1.55$$

$$b = 42.0 \text{ m} \quad \#$$

Q (2) :

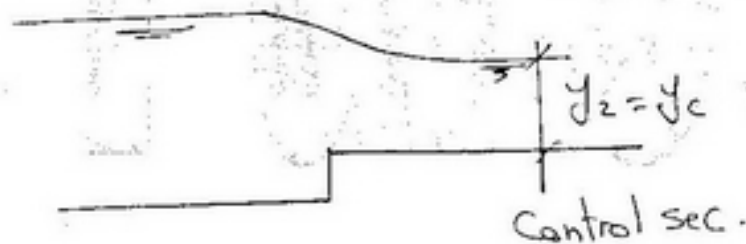
Control volume:

هو المنطقه المجرى لها التي يتم فيها عمل ايزان  
للتوى داخله



Control section

هو المقطع الذي يكون فيه اسريان فيه هو نفسه  
المخرج (اليد)



وعليه لا ستفاده منه في حساب الكون

State of flow:

کے تصنیف و تالیف (Rn)

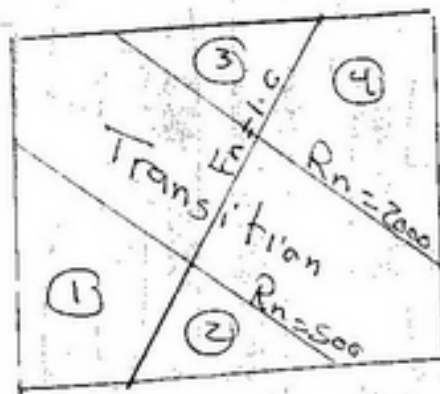
Turbulent  $R_n > 2000$

Laminar  $Re < 500$

Transition  $500 < R_n \leq 2000$

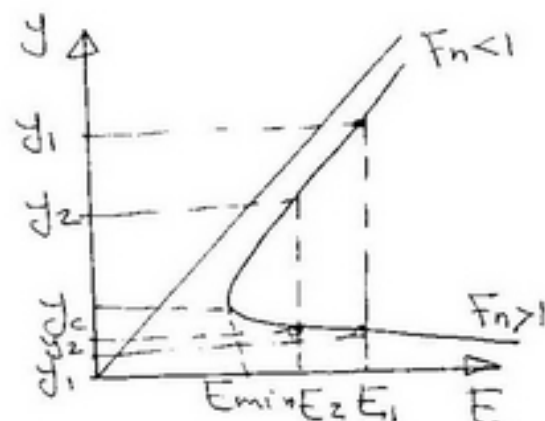
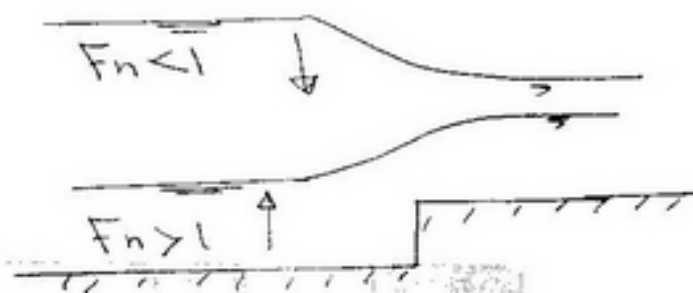
## Regimes of flow

60  $F_n$   $C_{R_n}$  de 100 cent

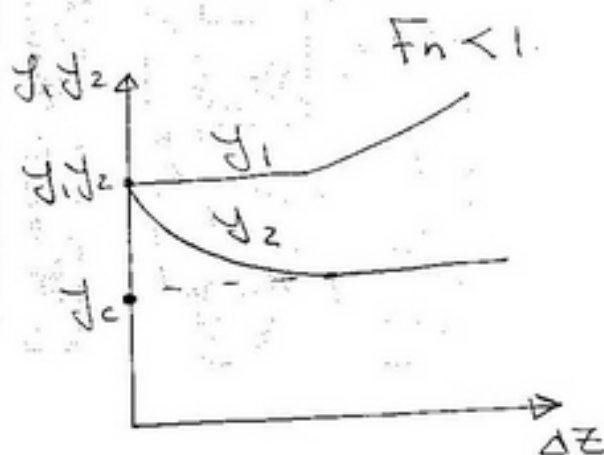
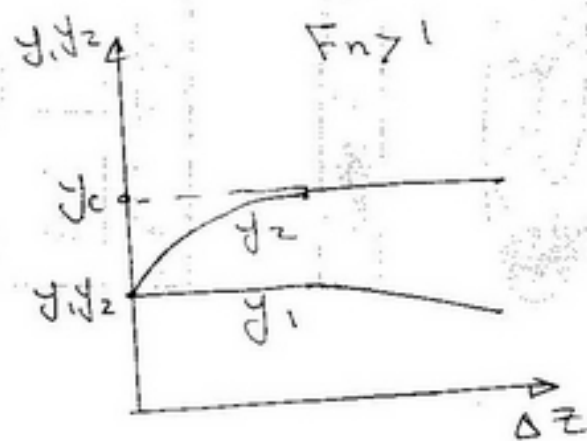
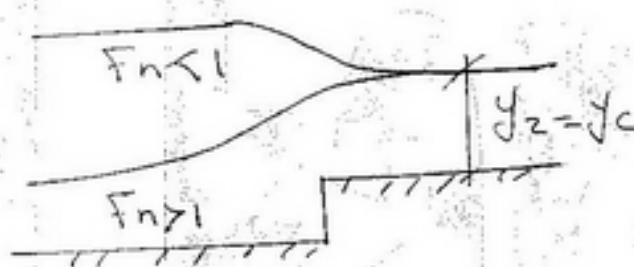


- ① Laminar - sub Critical
- ② " - super "
- ③ Turbulent - sub - "
- ④ " - super

(B):



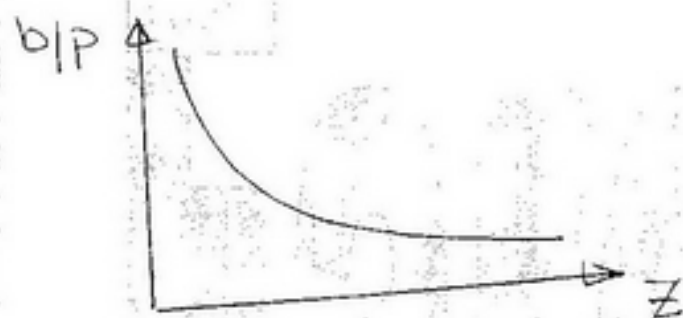
Limit:





(c) :

$\frac{b}{p}$  ,  $z$  for B.H.S of trapezoidal



$$R = \frac{y}{z} = \frac{A}{p} = \frac{(b + zy)y}{b + zy\sqrt{1+z^2}}$$

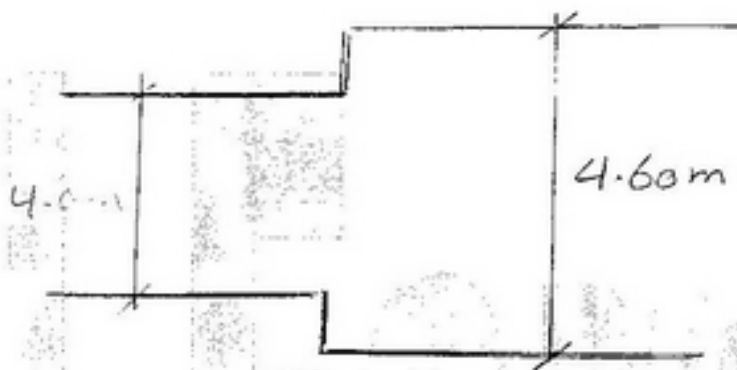
$$\frac{y}{z} = \frac{(b + zy)y}{p}$$

$$p = 2b + 2zy$$

$$\frac{p}{b} = 2 + \frac{2zy}{p} \Rightarrow \boxed{\frac{b}{p} = \frac{1}{2 + \frac{2zy}{p}}}$$

(d):

$$b = 4.0 \text{ m}$$

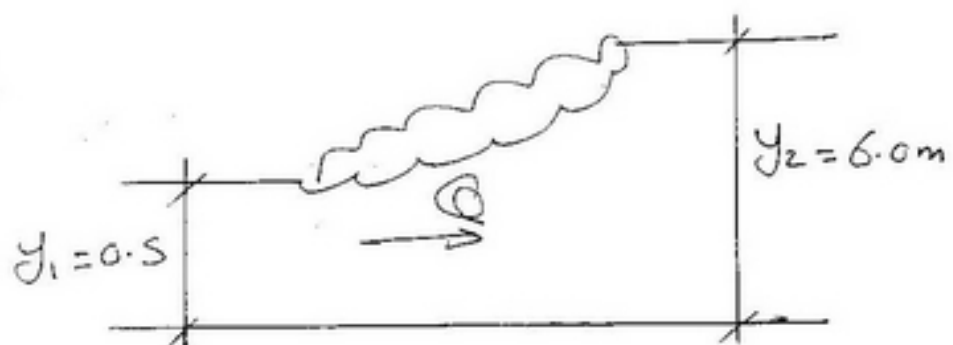


$$y_1 = 0.5 \text{ m}, \quad y_2 = 6.0 \text{ m}$$

Req.:

- $Q = ??$
- power dissipated
- $\eta$

Sol.:



$$P_1 + M_1 = P_2 + M_2$$

$$\gamma h_1' \cdot A_1 + \frac{\gamma Q^2}{g A_1} = \gamma h_2' \cdot A_2 + \frac{\gamma Q^2}{g A_2}$$

$$A_1 = 4 \times 0.5 = 2 \text{ m}^2$$

$$h_1' = \frac{y_1}{2} = \frac{0.5}{2} = 0.25$$

$$A_2 = 4.6 \times 6 = 27.6 \text{ m}^2$$

$$h_2' = \frac{y_2}{2} = \frac{6}{2} = 3.0$$

$$1 \times 0.25 \times 2 + \frac{1 \times Q^2}{9.81 \times 2} = 1 \times 3 \times 27.6 + \frac{1 \times Q^2}{9.81 \times 27.6}$$

$$Q = 41.72 \text{ m}^3/\text{s} \quad \#$$

$$E_1 = y_1 + \frac{V_1^2}{2g}$$

$$V_1 = \frac{41.72}{2} = 20.86 \text{ m/s}$$

$$E_1 = 0.5 + \frac{20.86^2}{2 \times 9.81} = 22.68 \text{ m}$$

$$E_2 = y_2 + \frac{V_2^2}{2g}$$

$$V_2 = \frac{Q}{A_2}$$

$$= \frac{41.72}{27.6} = 1.52 \text{ m/s}$$

$$E_2 = 6 + \frac{(1.52)^2}{2 \times 9.81} = 6.12 \text{ m}$$

$$\eta = \frac{E_2}{E_1} = \frac{6.12}{22.68} = 0.2698 \#$$

$$h_L = 22.68 - 6.12 = 16.56 \text{ m}$$

$$\text{Power} = \frac{\gamma Q h}{75 \times \eta}$$

$$= \frac{1000 \times 41.72 \times 16.56}{75 \times 0.2698}$$

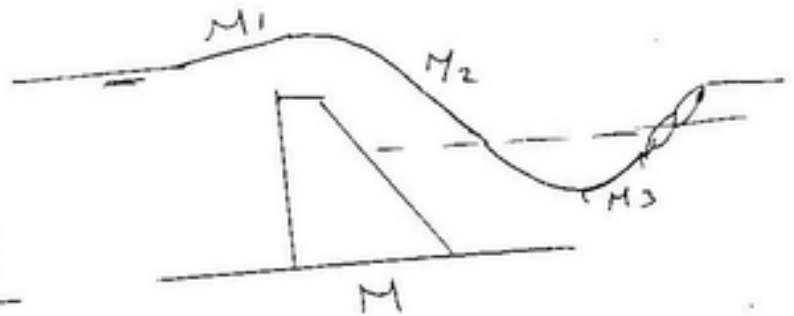
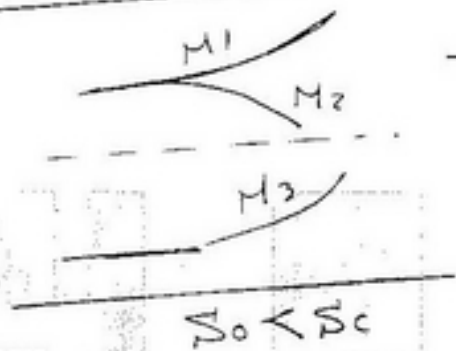
$$\text{Power} = \quad \quad \quad \text{H.P. \#}$$


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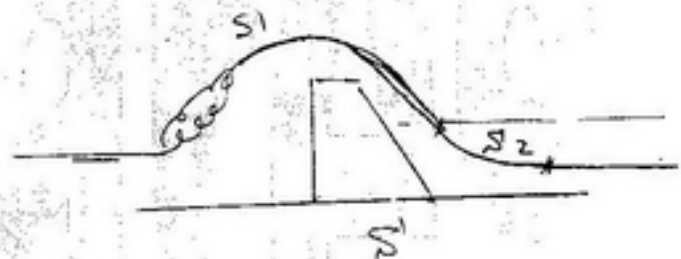
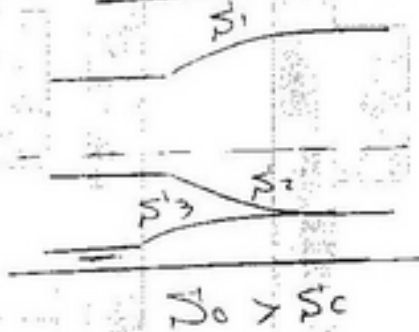
Q (3):

(A):

(1) Mild slope



(2) Steep slope.



(3) Critical

(4) Horizontal

(5) adverse

JKI

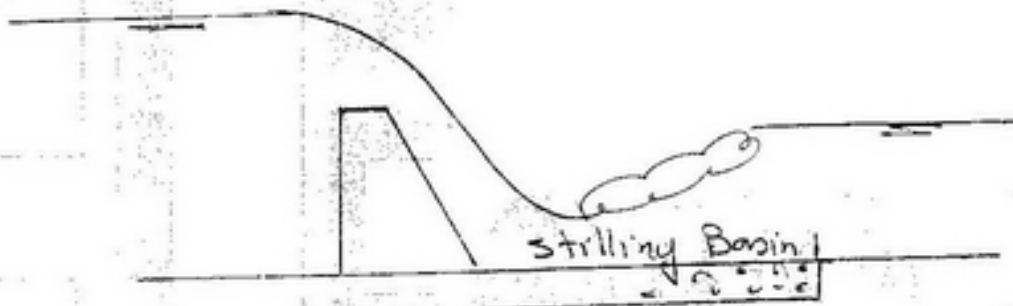


(B) :

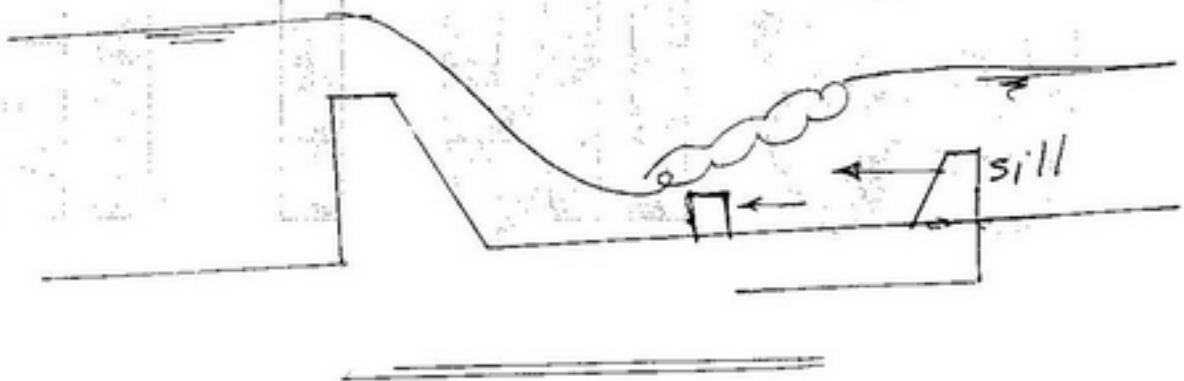
Stilling Basin:

حوض تهدئة

هذه المنطقة من لقناة التي يتم فيها تثبيت جزء  
من طاقة الحركة لتهدئته، بسرطان و يورده  
خلف المنشآت -



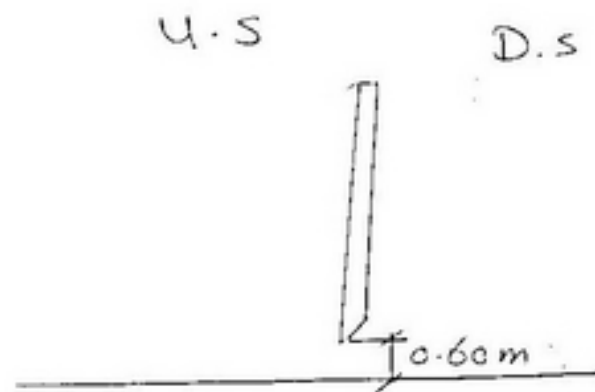
to control location of H.J use sill



(C):

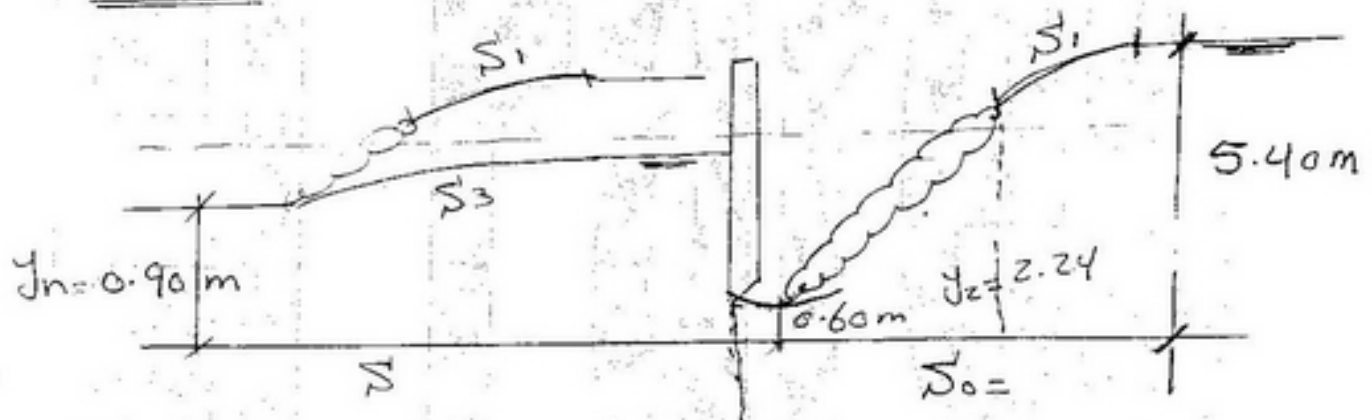
$$Y_n = 0.90 \text{ m}$$

$$V = 4.8 \text{ m/s}$$



Req.: a - draw and name water profile  
b - D.S water depth = 0.8 m/s  
 $n = 0.025$

Sol.:



$$\therefore Y_c = \sqrt[3]{q^2/g}$$

for unit width

$$q = \frac{Q}{b} = Q$$

$$\therefore Q = A \times V$$

$$q = Q = 4.8 \times (0.9 \times 1.0) = 4.32 \text{ m}^3/\text{s}$$

$$y_c = \sqrt[3]{\frac{4.32^2}{9.81}} = 1.23 \text{ m}$$

$$y_n < y_c$$

for D.S.

$$Q = A \times V$$

$$4.32 = A \times 0.8$$

$$A = 5.4 \text{ m}^2$$

$$5.4 = b \times y$$

$$y = 5.40 \text{ m}$$

$$\therefore y_1 = 0.60 \text{ m}$$

$$\therefore \frac{y_2}{y_1} = 0.5 \left[ \sqrt{1 + 8F_{n1}^2} - 1 \right]$$

$$\therefore F_{n1} = \frac{V_1}{\sqrt{g \cdot y_1}}$$

$$V_1 = \frac{Q}{A_1} = \frac{4.32}{1 \times 0.6} = 7.2$$

$$F_{n1} = \frac{7.2}{\sqrt{9.81 \times 0.6}} = 2.97$$

$$\therefore \frac{y_2}{0.6} = 0.5 \left[ \sqrt{1 + 8 \times 2.97^2} - 1 \right]$$

$$y_2 = 2.24 \text{ m}$$

$$\Delta x = \frac{\Delta E}{\Delta S}$$

$$\Delta E = E_2 - E_1$$

$$\Delta S = S_0 - (SE)_{av.}$$

$$\therefore Q = \frac{1}{n} \cdot \frac{A^{5/3}}{P^{2/3}} \cdot S^{1/2}$$

$$A = 5.4 \times 1 = 5.4 \text{ m}^2$$

$$P = 5.4 \times 2 + 1 = 11.8$$

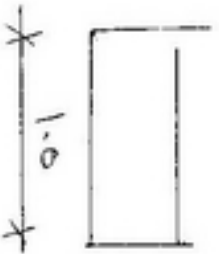
$$4.32 = \frac{1}{0.025} \times \frac{(5.4)^{5/3}}{(11.8)^{2/3}} \times S^{1/2}$$

$$S_0 = 0.0011$$

$$E = y + \frac{V^2}{2g}$$

$$N_E = \frac{N^2 \cdot V^2}{R^{4/3}}$$

$$n = 0.025$$



AIP

So -  $\Sigma E_{av}$

Sec No	y	A	V	E	$\Delta E$	P	R	$\Sigma E$	$\Sigma E_{av}$	$\Delta S$	$\Delta x$
1	2.24	2.24	1.93	2.43		5.48	0.41	0.0076			
					3.0				0.0044	0.0033	909.1
2	5.4	5.4	0.8	5.43		11.8	0.46	0.0011			m
											#